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INSTITUTE OF ENVIRONMENTAL ENGINEERING

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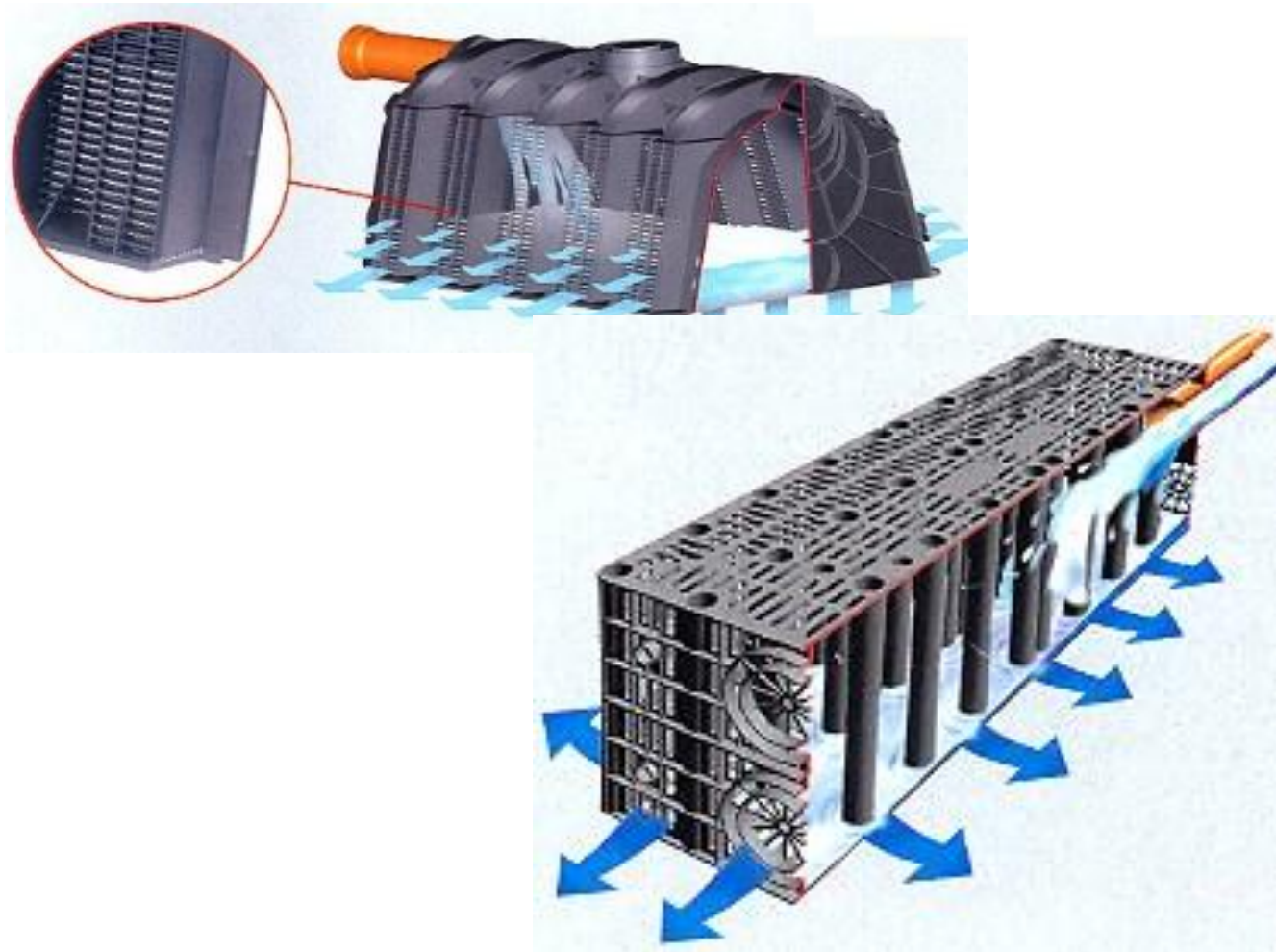
**MODELLING OF PERCOLATION RATE OF STORMWATER  
FROM UNDERGROUND INFILTRATION SYSTEMS**

*9th International Conference on Urban Drainage Modelling, Belgrade 2012*

**Subsurface stormwater storage tank systems with infiltration of water into the ground constitute the basic elements used in Sustainable Urban Drainage Systems (SUDS).**



# MECHANISMS OF OPERATION



The first model of infiltration of water into the ground was developed by Green and Ampt in the year 1911.

$$q(t) = \frac{dI(t)}{dt} = K_f \left( \frac{H_s + Z(t) - h_f}{Z(t)} \right) = \left( \frac{H_s - h_f}{Z(t)/K_f} \right) + K_f \quad (1)$$

$I(t)$  accumulated water infiltration into the ground (cm)

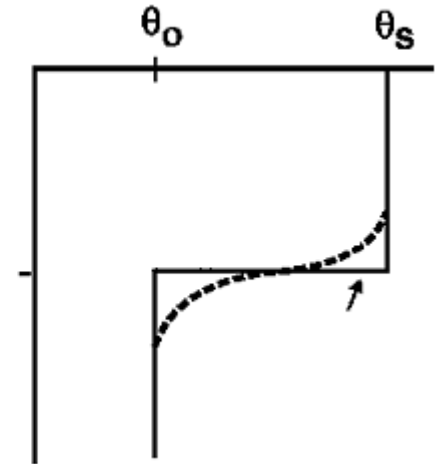
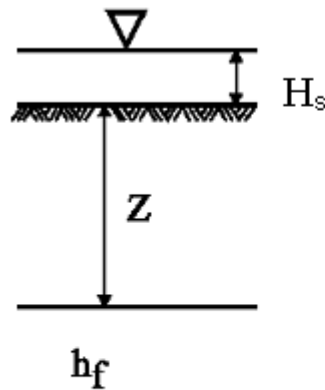
$t$  time (min)

$K_f$  wetting zone hydraulic conductivity (cm min<sup>-1</sup>),

$H_s$  water level above ground surface (cm)

$Z(t)$  depth of wetting front (cm)

$h_f$  negative pressure head at wetting front (cm)



Assuming that:

$$Z(t) = Z_{\text{const}} \quad (2)$$

and introducing a parameter  $R$  defined by the ratio:

$$R = Z_{\text{const}} / K_f \quad (3)$$

equation (1) will have the form:

$$q(t) = \frac{H_s - h_f}{R} + K_f \quad (4)$$

Where:

$Z(t)$  depth of wetting front (cm)

$K_f$  wetting zone hydraulic conductivity ( $\text{cm min}^{-1}$ ),

$q(t)$  infiltration rate at time  $t$  ( $\text{cm min}^{-1}$ ),

$h_f$  negative pressure head at wetting front (cm)

$R$  substituted hydraulic resistance (min)



## **THE AIM OF THE ANALYSES**

**develop simple models describing the changes in the infiltration rate in the phase of filling and emptying of retention and infiltration reservoirs and to test their usability in the evaluation of the progress of clogging.**

## CHARACTERISTICS OF THE MODEL

the increase in water volume  
in the inf.modules

volume of water flowing  
into the inf.modules

amount of water infiltrating  
into the ground

$$F \cdot dH(t) = Q \cdot dt - F \cdot q(t) \cdot dt \quad (5)$$

Assuming that:

$$q(t) = \frac{H(t)}{R} + K_f \quad (6)$$

and introducing a parameter defined by the ratio:

$$Q_F = \frac{Q}{F} \quad (7)$$

$$H(t) = Q_F R \left( 1 - \exp\left(-\frac{t}{R}\right) \right) - K_f R \left( 1 - \exp\left(-\frac{t}{R}\right) \right) \quad (8)$$

where:

$F$  bottom surface of infiltration facility ( $\text{cm}^2$ )

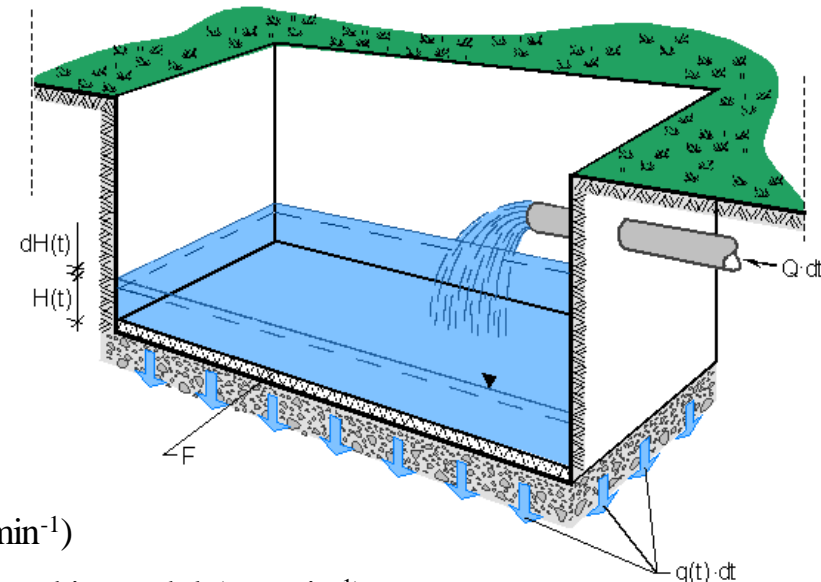
$H(t)$  water level in the infiltration module at time  $t$  (cm)

$Q$  infiltration flow ( $\text{cm}^3 \text{min}^{-1}$ ),

$q(t)$  infiltration rate at time  $t$  ( $\text{cm min}^{-1}$ ),

$t$  time (min),  $K_f$  wetting zone hydraulic conductivity ( $\text{cm min}^{-1}$ )

$R$  substituted hydraulic resistance (min),  $Q_F$  variable parameter used in model ( $\text{cm min}^{-1}$ ),



The function of change of the water surface level in the module in the emptying phase will be described by the following equation:

$$H(t) = H_0 \exp\left(-\frac{t}{R}\right) - K_f R \left(1 - \exp\left(-\frac{t}{R}\right)\right) \quad (9)$$

During the infiltration of water containing suspension a significant decrease of the filtration coefficient of the top layer of soil is observed. In the case if the value of the product of the  $K_f \cdot R$  constants is close to 0, equations (8) and (9) will have the respective forms:

$$H(t) = Q_F R \left(1 - \exp\left(-\frac{t}{R}\right)\right) \quad (10)$$

$$H(t) = H_0 \exp\left(-\frac{t}{R}\right) \quad (11)$$

where:

$H(t)$  water level in the infiltration module at time  $t$  (cm)

$H_0$  water level in the infiltration module at the end of the filling phase (cm)

$t$  time (min),

$K_f$  wetting zone hydraulic conductivity ( $\text{cm min}^{-1}$ )

$R$  substituted hydraulic resistance (min),

$Q_F$  variable parameter used in model ( $\text{cm min}^{-1}$ ),



## EXPERIMENTAL SITE

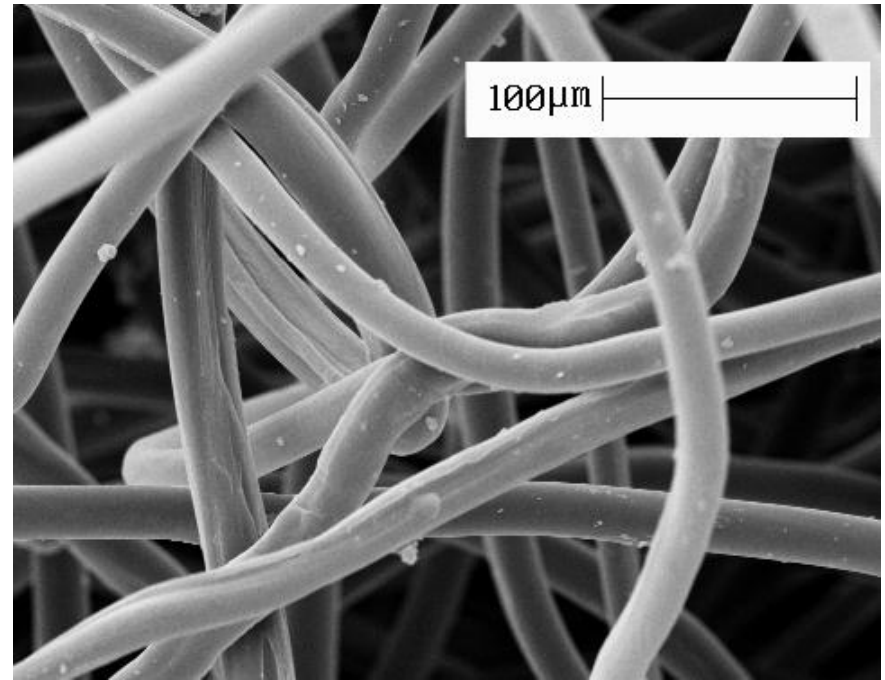


The dimensions of the inf. modules  
are 500x1000x400 mm

commonly used in  
engineering practice !!!



# GEOTEXTILE

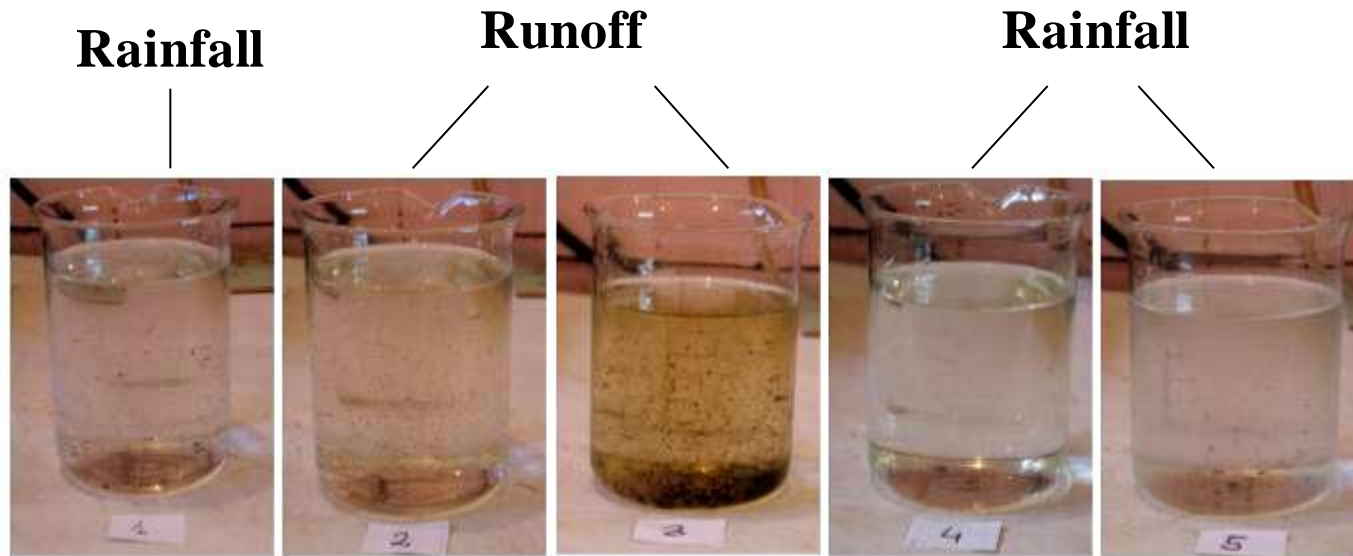


Scanning photos of geotextile magnified 1000 times

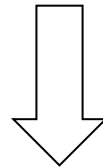
# LITHOLOGICAL PROFILES OF THE SOIL

BOREHOLE No.1				BOREHOLE No.2		
SCALE	DEPTH OF SOIL	TYPES OF SOIL	EXPERIMENTAL SETUP	SCALE	DEPTH OF SOIL	TYPES OF SOIL
0,20	0,20	humus		0,20	0,20	humus
0,40	0,60	medium sand		0,20	0,20	fine sand
0,60				0,50	sandy clay	
0,80						
1,00	0,50	sandy clay		1,00	0,40	cohesive clay
1,20						
1,40	0,40	medium sand		1,40	1,10	sandy loam
1,60						
1,80						
2,00	0,70	fine sand		2,00	0,20	sandy clay
2,20						
2,40	0,20	sandy clay	2,40			
2,60						

## SAMPLES OF RAINFALL AND RUNOFF



1-rainfall -11.06.2004, Strzelin, Jana Pawła II; 2-runoff - 11.06.2004, Strzelin Jana Pawła II; 3- runoff -9.06.2004 Wrocław, Dicksteina; 4- rainfall - 9.06.2004, Bielawa Oś. XXV-lecia; 5- rainfall 10.04.2004, Racibórz

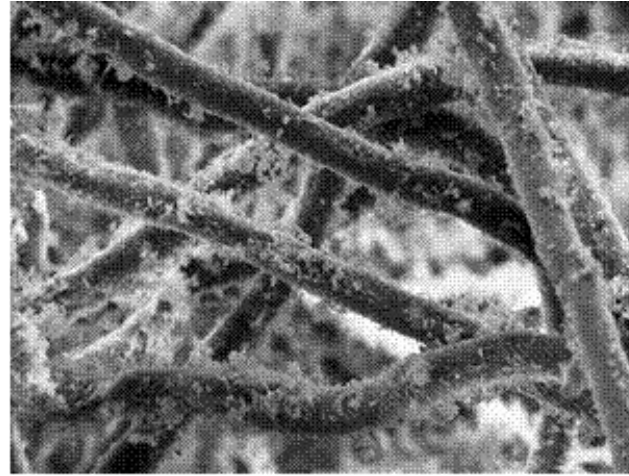
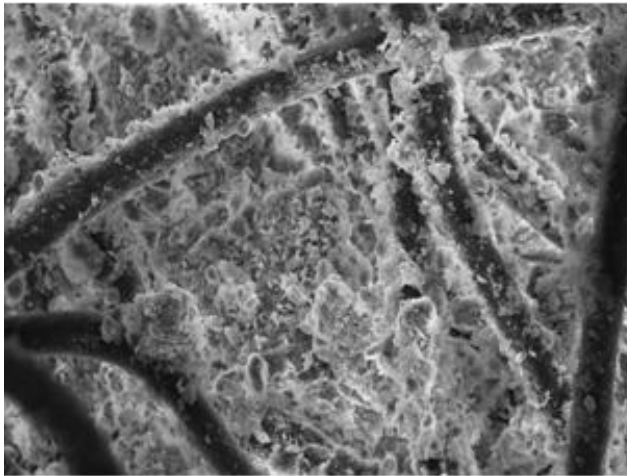


**kaolin clay suspension  
(clogging agent)**

## Results



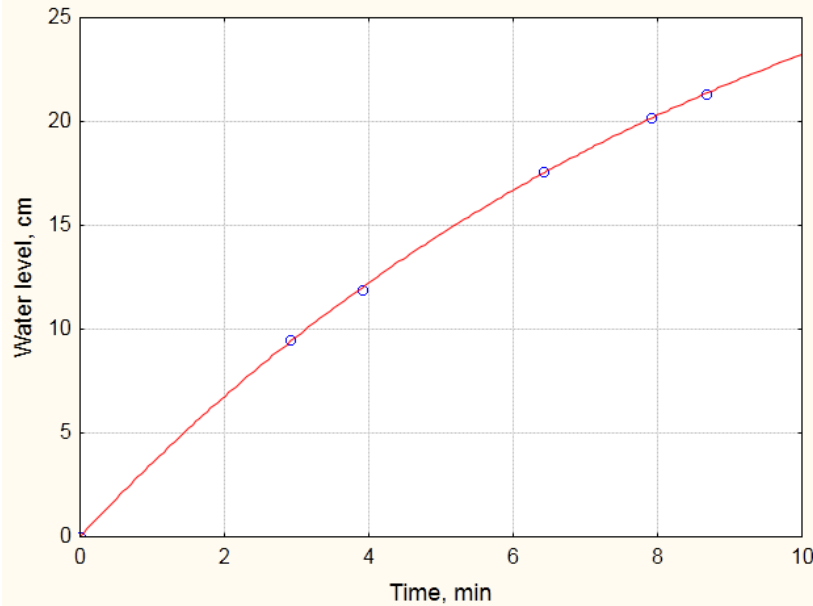
The appearance of the module (site 2) after removal from the ground and the distribution of sediments on geotextile on the inside



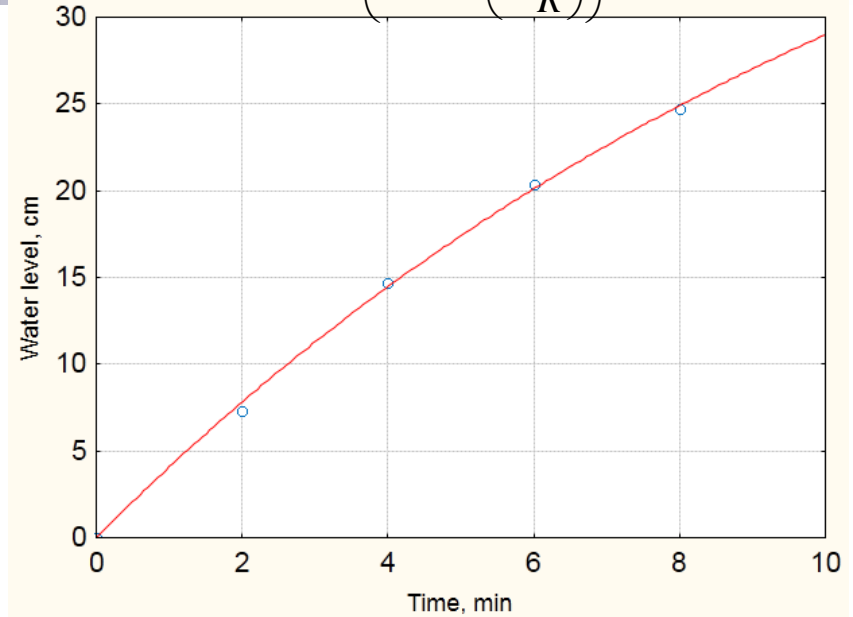
Scanning photos of kaolin clay sediment deposited in the geotextile on the bottom of the module (left image) and on the side walls of the site (right image) magnified 1000 times

# Changes in the water level during filling

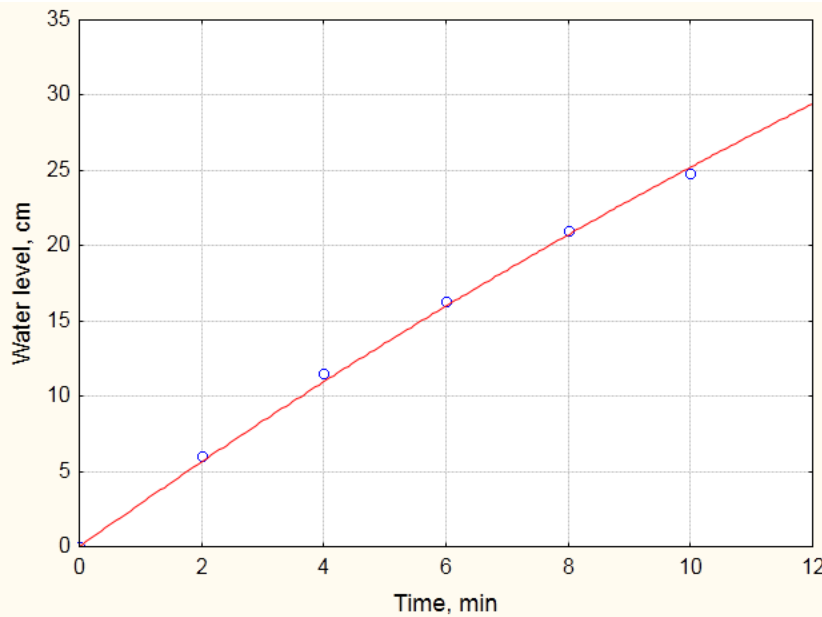
$$H(t) = Q_F R \left( 1 - \exp\left(-\frac{t}{R}\right) \right)$$



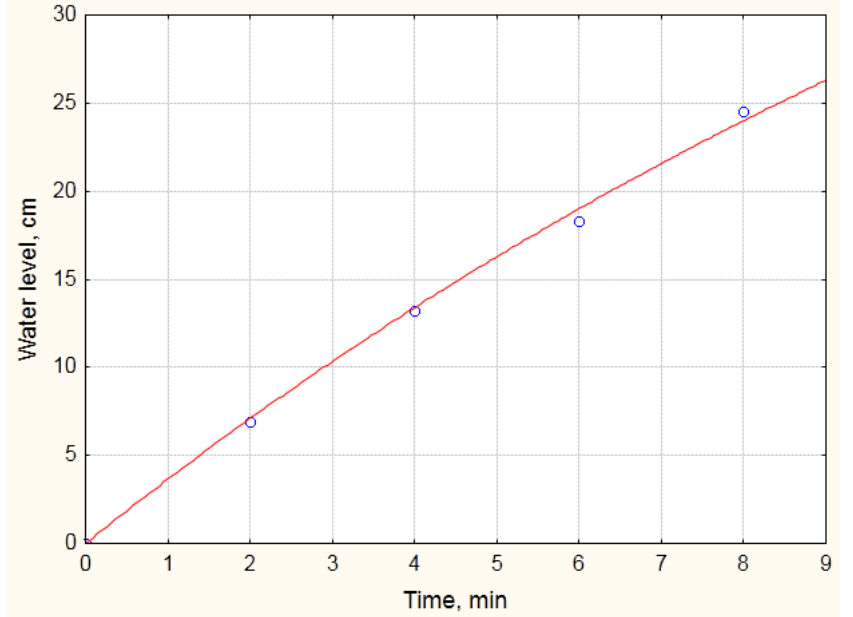
15.09.03 ( $R^2=0,999$ )



10.12.03 ( $R^2=0,995$ )



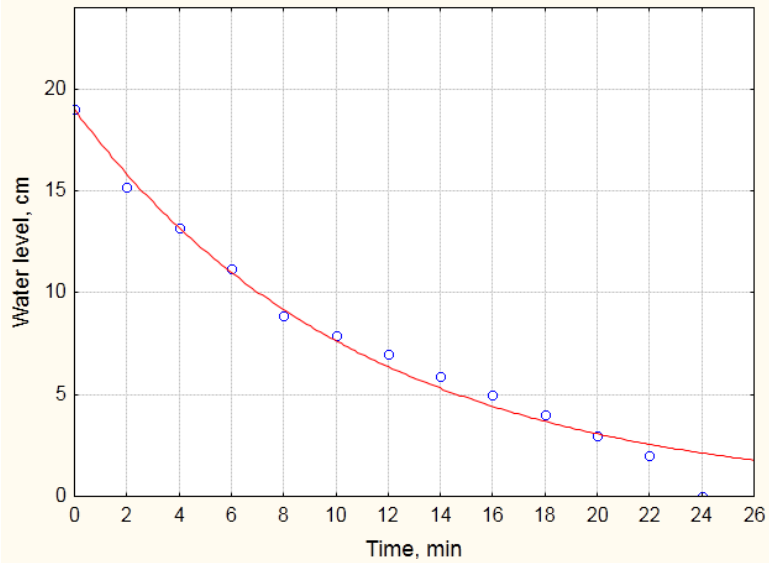
11.07.03 ( $R^2=0,998$ )



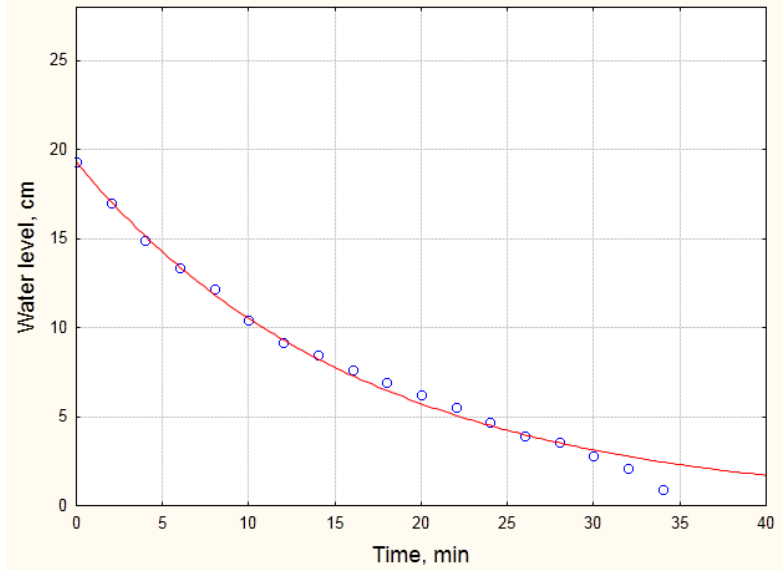
18.08.03 ( $R^2=0,989$ )

# Changes in the water level during infiltration

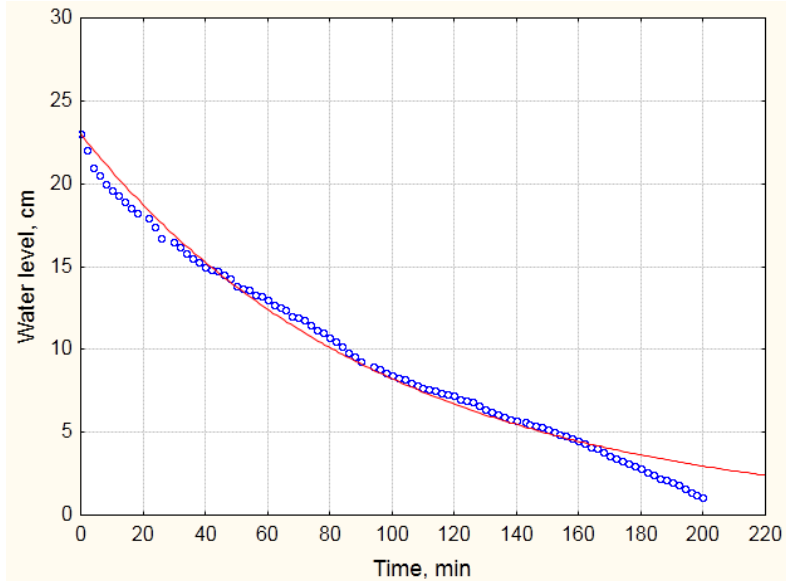
$$H(t) = H_0 \exp\left(-\frac{t}{R}\right)$$



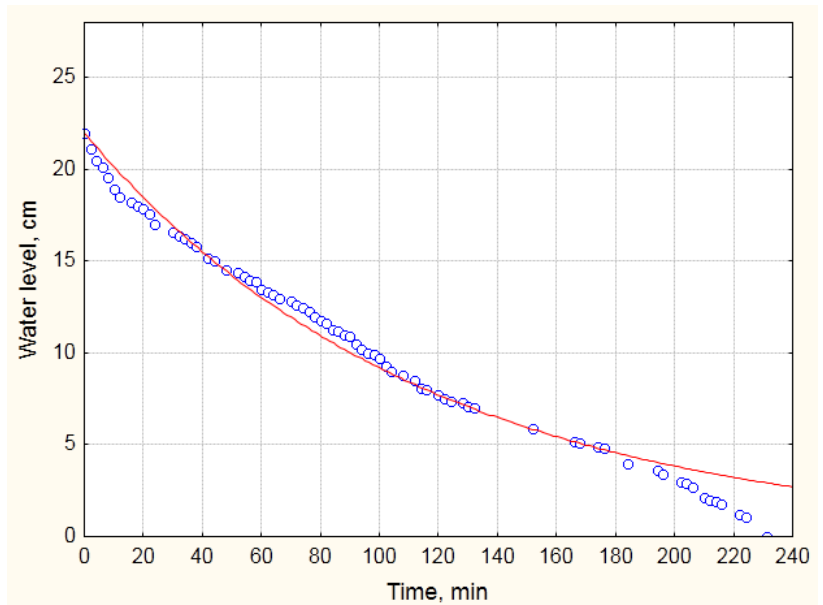
30.07.03 ( $R^2 = 0,964$ )



11.08.03 ( $R^2 = 0,985$ )



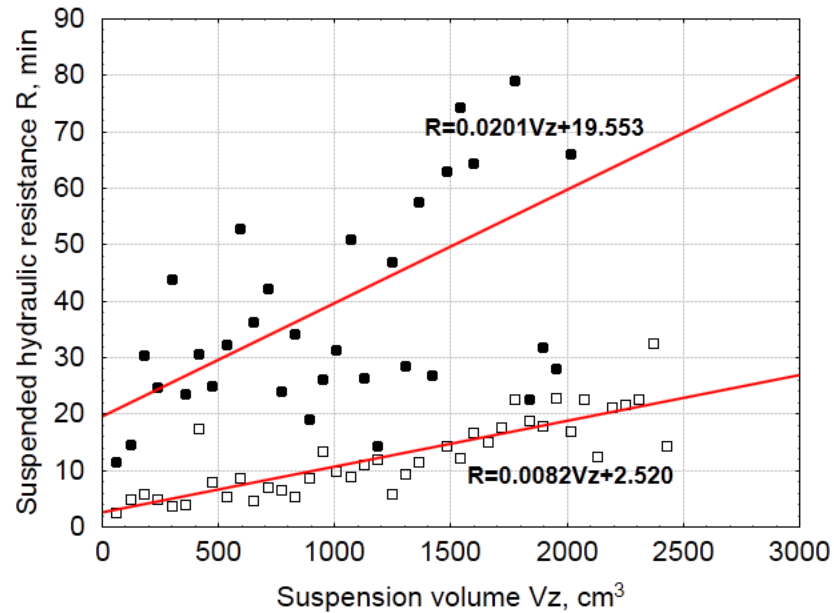
09.07.03 ( $R^2 = 0,978$ )



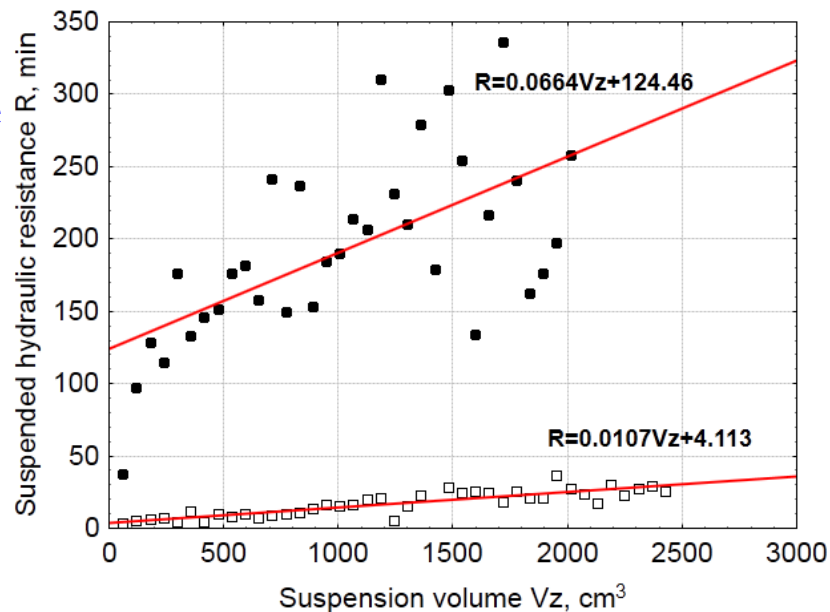
14.07.03 ( $R^2 = 0,971$ )

# Hydraulic resistance of soil (R) on experimental site no.1 (white points) and experimental site no. 2 (black points)

- in the filling phase



- in the infiltration phase





# CONCLUSIONS

1. **Complex models of soil clogging have a limited practical value for the purposes of designing of stormwater infiltration facilities.** This results from a large variability of the conditions in which the infiltration process takes place and from the complexity of the formal description of the models.
2. For engineering purposes, **simple models of changes in the infiltration rate in the phase of filling and emptying of infiltration modules can be applied.** The practical value of the developed models for the designing of stormwater infiltration systems translates mainly in the practical possibility to evaluate the degree of clogging and the need to take declogging actions.
3. **Substitute hydraulic resistance  $R$  of the soil in the phase of filling of infiltration modules is significantly lower than that in the emptying phase. The difference between these values increased along with the increase in the volume of suspension introduced with infiltrating water,** which can be explained by a gradual clogging of the geotextile and of the superficial layer of soil.
4. **The degree and rate of clogging of the ground** in the course of long-term exploitation of stormwater storage and infiltration facilities **can be determined** basing on the measured values with use of **substitute hydraulic resistance  $R$**  in the emptying phase.