

Modelling Internal Boundary Conditions of a Sewer Network

Nuno Melo, Jorge Leandro, James Shucksmith, Matteo Rubinato, Slobodan Djordjevic, Adrian J. Saul, Helena Ramos, João L. M. P. de Lima



University of Coimbra



University of Sheffield



University of Exeter



IMAR
Institute of Marine Research



UDI
Research Unit for Inland
Development

Structure of the Presentation

Introduction

Introduction

Methodology

Methodology

Physical Model

Physical Model

Hydraulic Numerical Models

Hydraulic Numerical Models

Data Analysis

Data Analysis

Results and Discussion

Results and Discussion

Conclusion

Conclusion

Structure of the Presentation

Introduction

Methodology

Physical Model

Hydraulic
Numerical
Models

Data Analysis

Results and
Discussion

Conclusion



Flooding in Manila - Philippines



Flooding in UK - 2012



Sewer system flooding

Structure of the Presentation

Introduction

The aim of the work is to validate the internal boundary conditions of a scale model of an urban drainage system.

Methodology

Physical Model

Hydraulic Numerical Models

Data Analysis

Results and Discussion

Conclusion

The facility is a scale model of an urban drainage system.

The calibration of the models is done using the experimental data of two storm events.

Structure of the Presentation

Introduction

Methodology

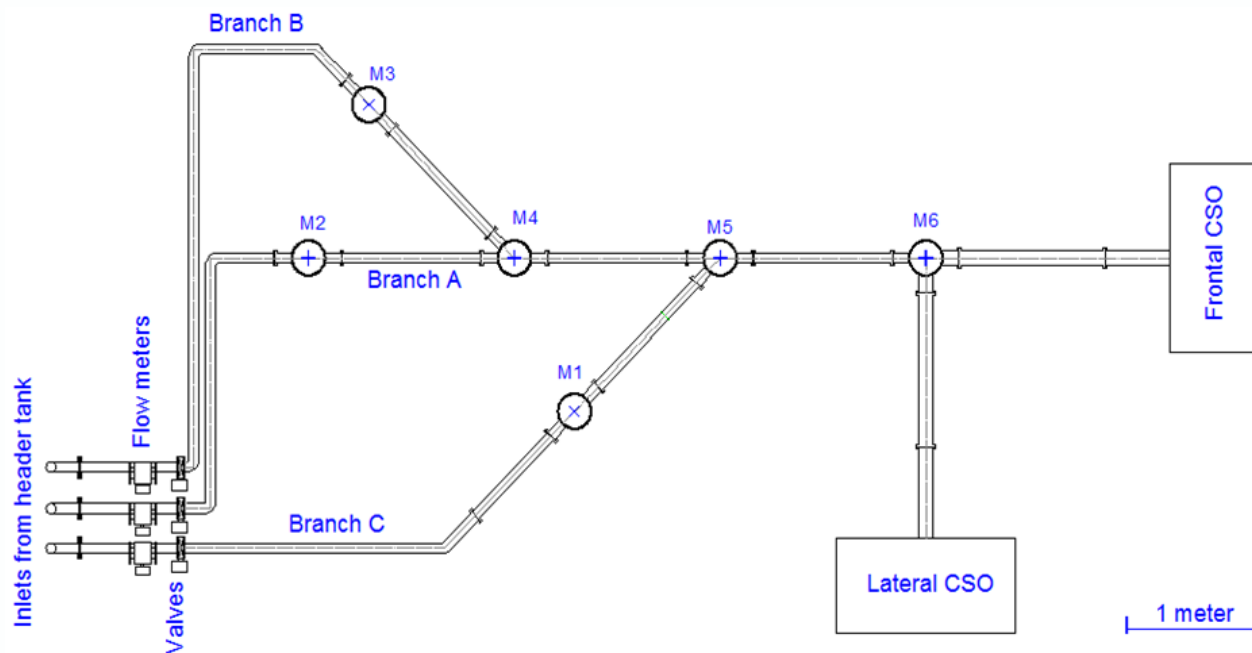
Physical Model

Hydraulic
Numerical
Models

Data Analysis

Results and
Discussion

Conclusion



Scheme of the facility (M stands for manhole)

Physical Model

Structure of the Presentation

Introduction

Methodology

Physical Model

Hydraulic
Numerical
Models

Data Analysis

Results and Discussion

Conclusion

SIPSON – “Simulation of Interaction between Pipe flow and Overland flow in Networks”.

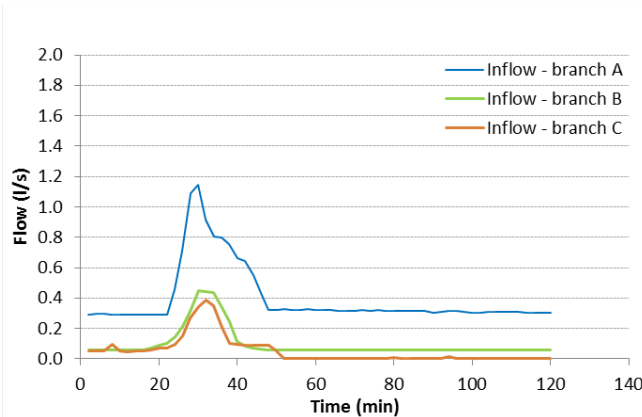
Djordjevic (2001)

SWMM – “Storm Water Management Model”.

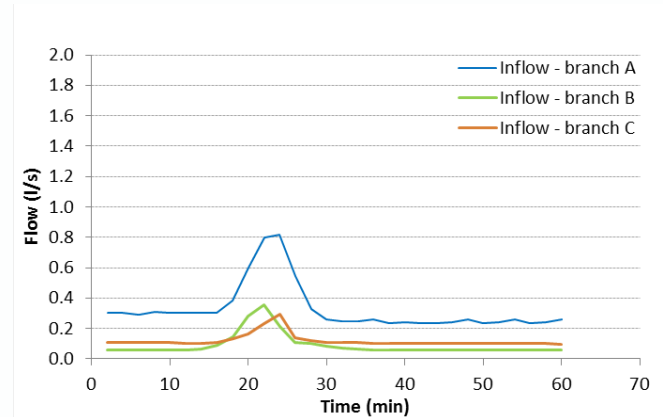
Rossman (2010)

Hydraulic Numerical Models

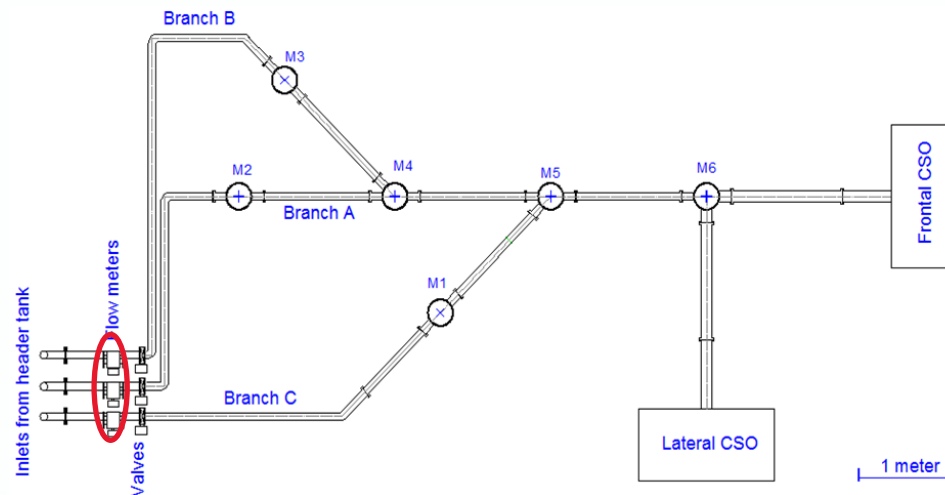
Inflow hydrographs at each inlet pipe



Event of 12th December 2008



Event of 17th January 2009



Data Analysis

Structure of the Presentation

Introduction

Methodology

Physical Model

Hydraulic Numerical Models

Data Analysis

Results and Discussion

Conclusion

Structure of the Presentation

Introduction

Methodology

Physical Model

Hydraulic Numerical Models

Data Analysis

Results and Discussion

Conclusion

It was found that the flow Froude number is always less than unity irrespectively of the event, indicating that we are in the presence of subcritical flow.

According to Zhao et al. (2006) subcritical flow in sewer junctions has relatively small energy losses.

In SIPSON the head losses considered in manholes was Special Type 3 (least energy loss considered in SIPSON).

In SWMM, the head losses in manholes are calculated considering losses coefficients on their inlet and outlet pipes.

Structure of the Presentation

Introduction

Methodology

Physical Model

Hydraulic

Numerical

Model

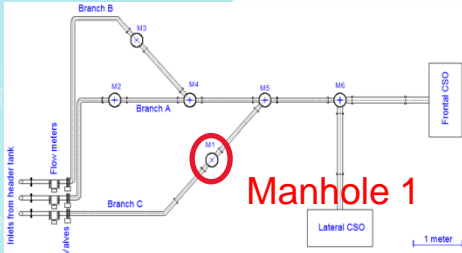
Results and Discussion

Conclusions

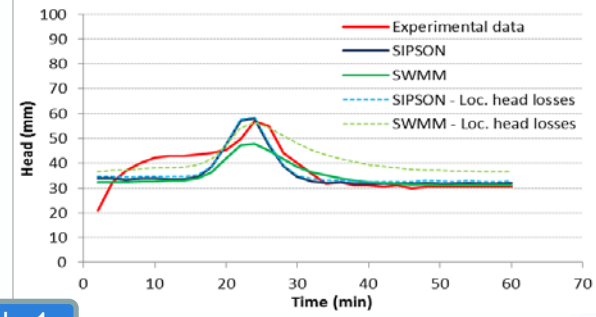
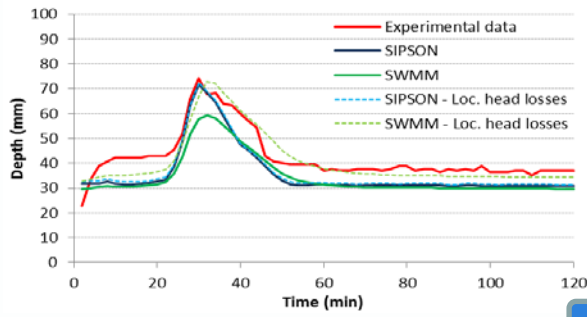
Variation of water depth

12th December 2008

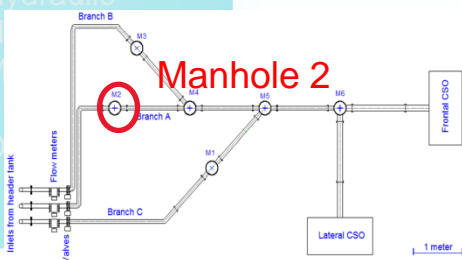
17th January 2009



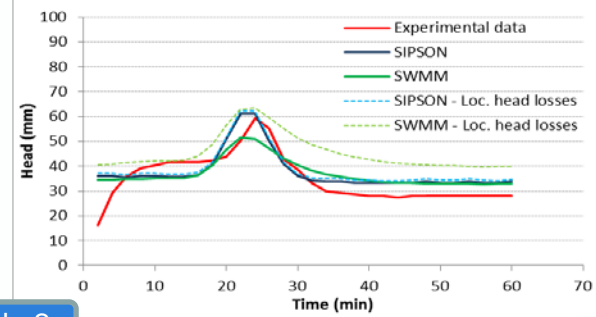
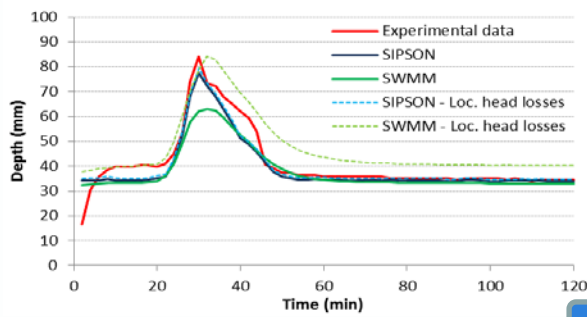
Manhole 1



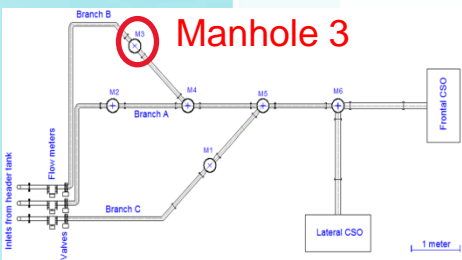
Manhole 1



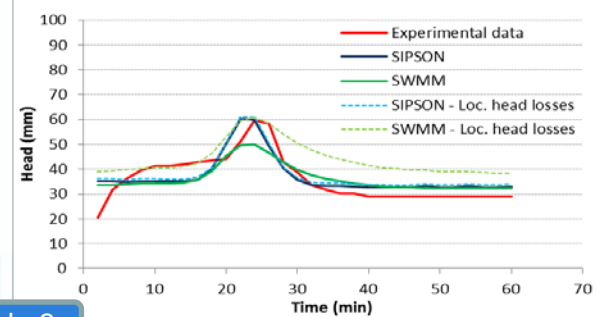
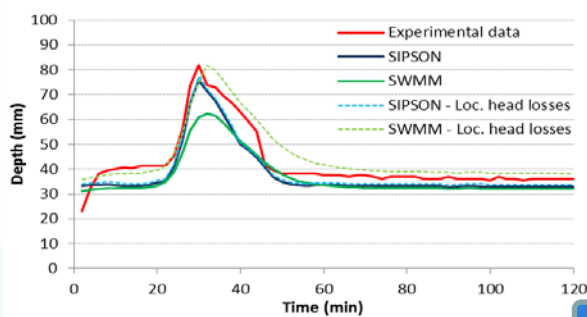
Manhole 2



Manhole 2



Manhole 3

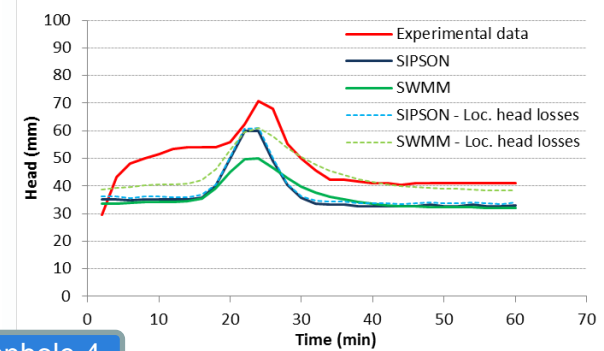
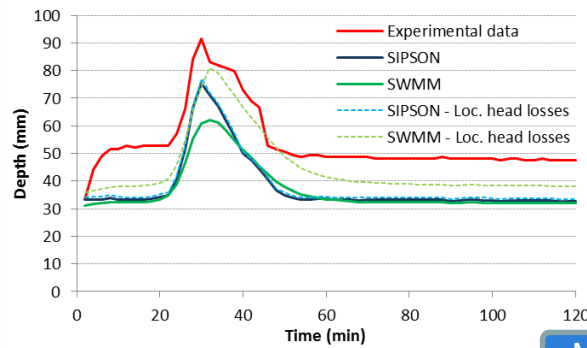
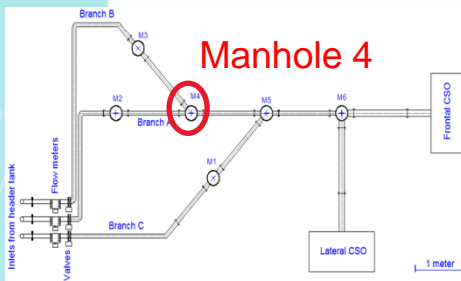


Manhole 3

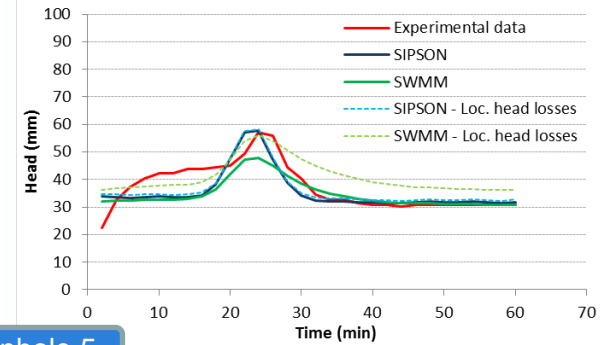
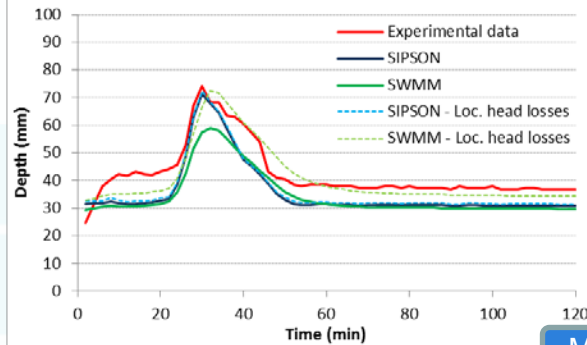
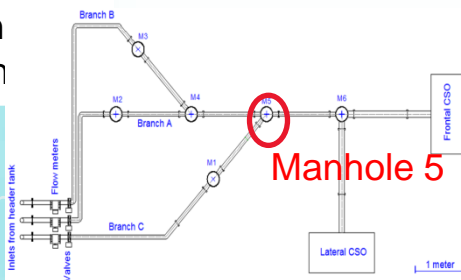
Variation of water depth

12th December 2008

17th January 2009



Manhole 4



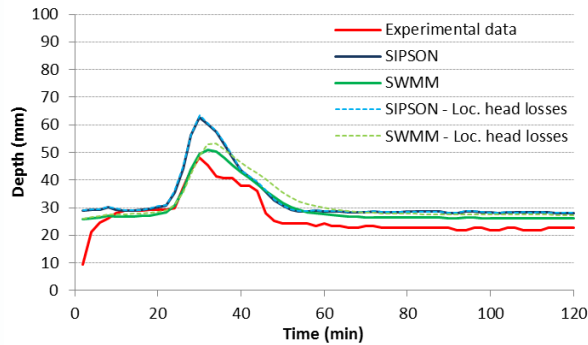
Manhole 5

Structure of the Presentation
 Introduction
 Methodology
 Physical Models
 Hydraulic Numerical Models
 Data Analysis

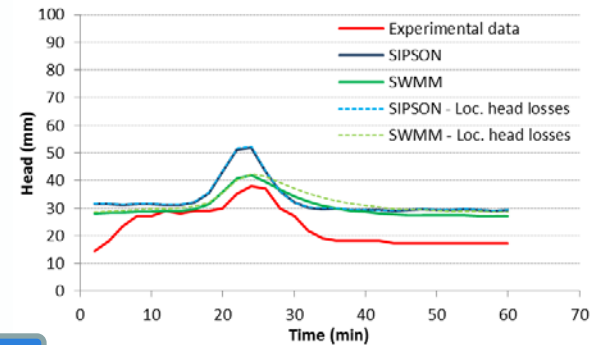
Results and Discussion
 Conclusion

Variation of water depth

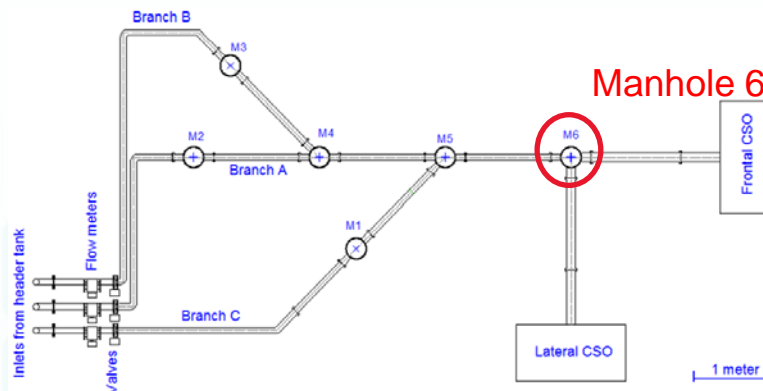
12th December 2008



17th January 2009

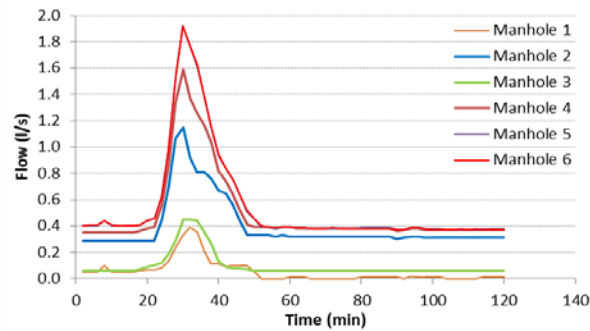


Manhole 6

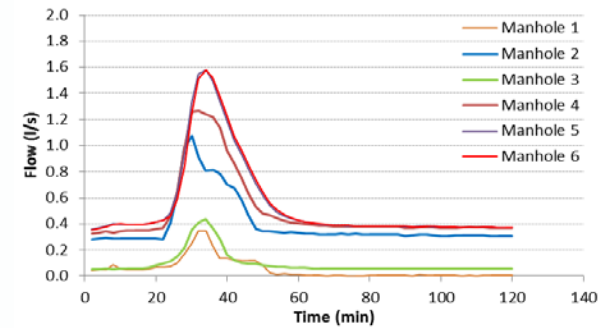


Variation of flow rate at manholes - event of 12th December 2008

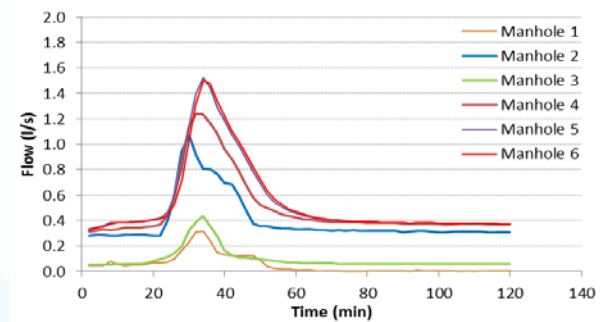
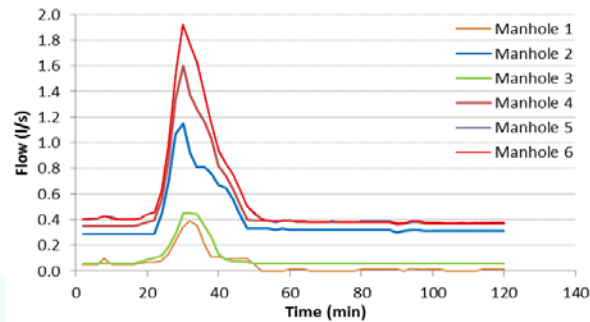
SIPSON results



SWMM results



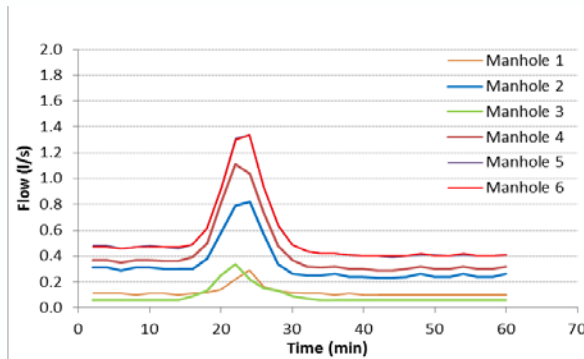
without considering head losses in manholes



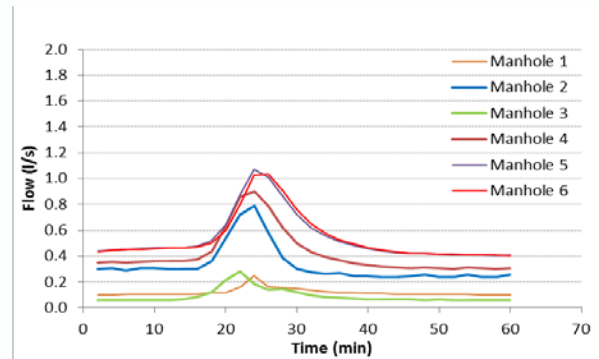
considering head losses in manholes

Variation of flow rate at manholes - event of 17th January 2009

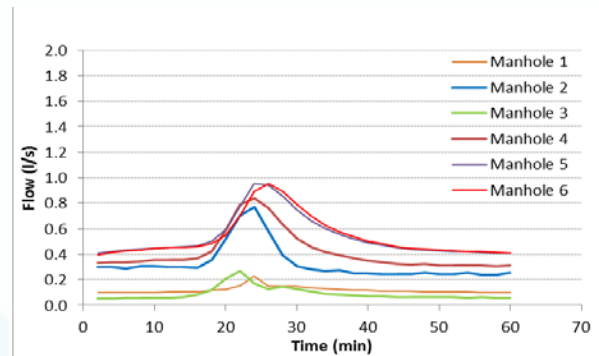
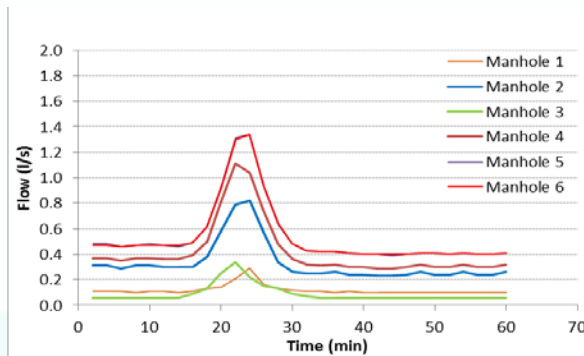
SIPSON results



SWMM results



without considering head losses in manholes



considering head losses in manholes

Structure of the Presentation

Introduction

Methodology

Physical Model

Hydraulic Numerical Models

Data Analysis

Results and Discussion

Conclusion

Structure of the Presentation

Introduction

Methodology

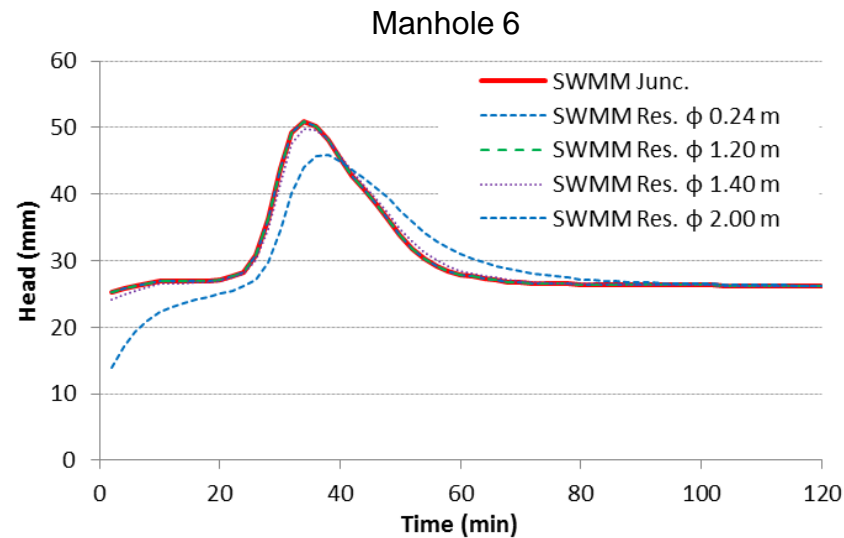
Physical Model

Hydraulic
Numerical
Models

Data Analysis

Results and Discussion

Conclusion



Comparison of the variation of water depth for two different simulations (with junctions or reservoirs to modelling manholes in SWMM).

Structure of the Presentation

Introduction

Methodology

Physical Model

Hydraulic
Numerical
Models

Data Analysis

Results and Discussion

Conclusion

SIPSON calculates the flow depth in manholes taking into consideration the depth increase due to transfer of kinetic energy to potential energy.

SWMM Flow depth inside manholes is equal to the average between the pipes upstream and downstream values.

In this experimental facility SIPSON reproduced fairly well the water depths in the manholes.

Thank you for your attention

Nuno Melo

Email contact: nuno_melo@ipg.pt



University of Coimbra
Civil Engineering Department



IMAR
Institute of Marine Research