

The Use of Natural Geological Formations to Decrease Storm Runoff

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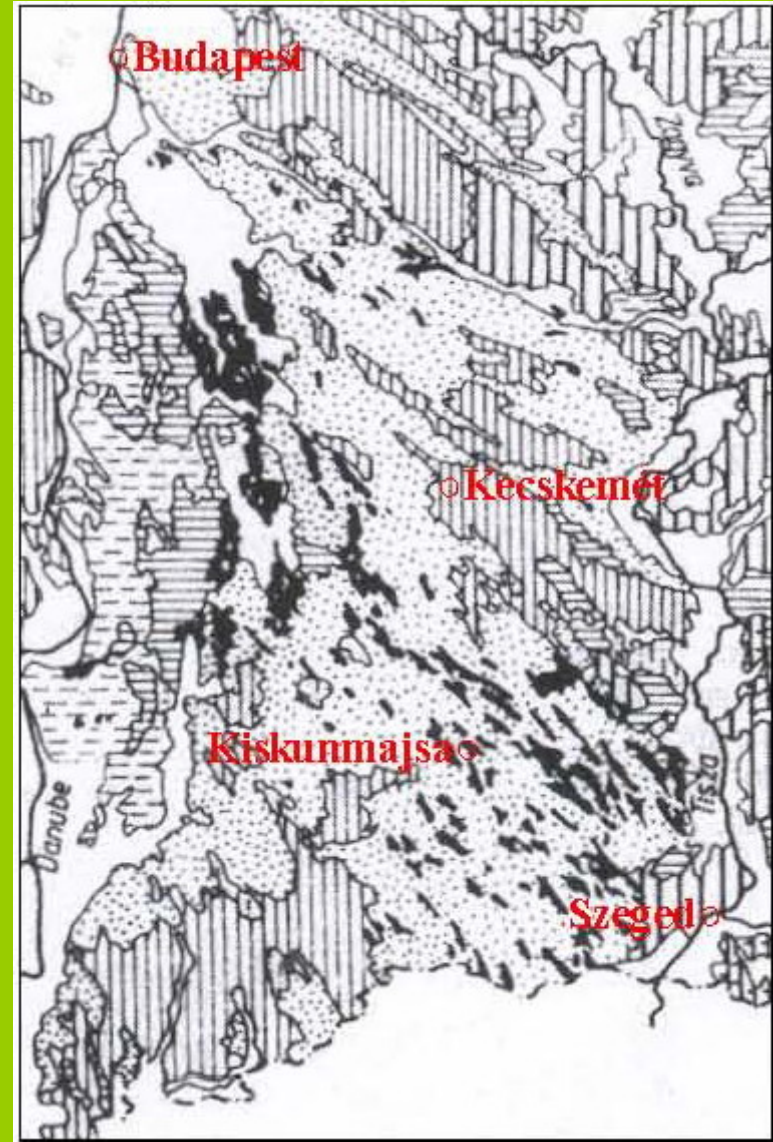
INTRODUCTION

1 and 2 – sand

impermeable floor
valleys:

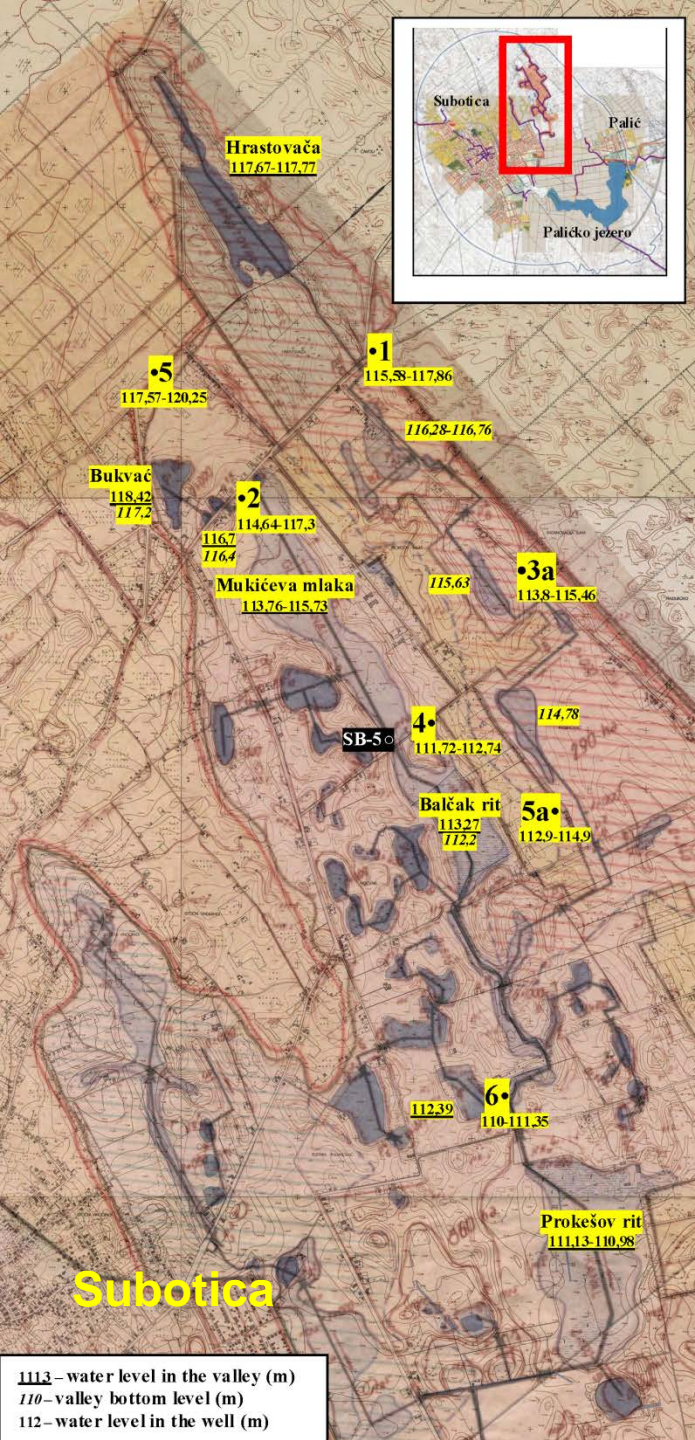
overall area 300 km²

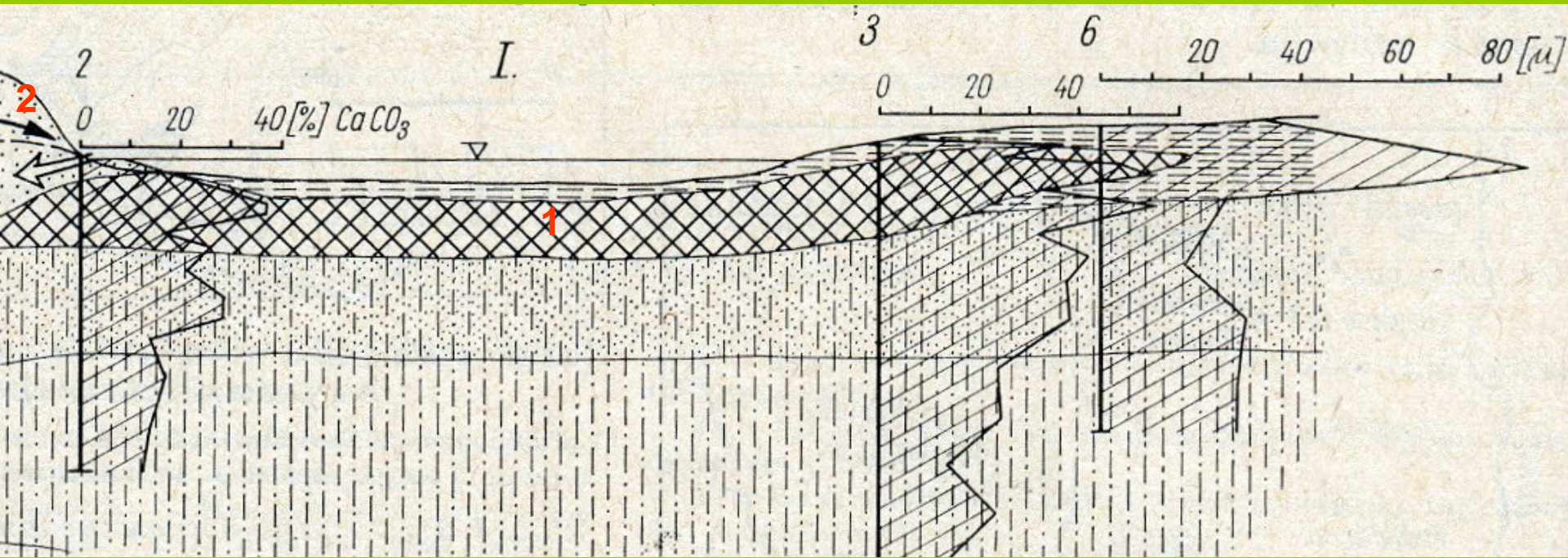
1,5 km² is in the
vicinity of Subotica



in type A valleys (dark blue areas): the water had uniform level

in type B valleys (light blue areas): the water level was not uniform





1 impermeable material: dolomite or a white, pulpy material

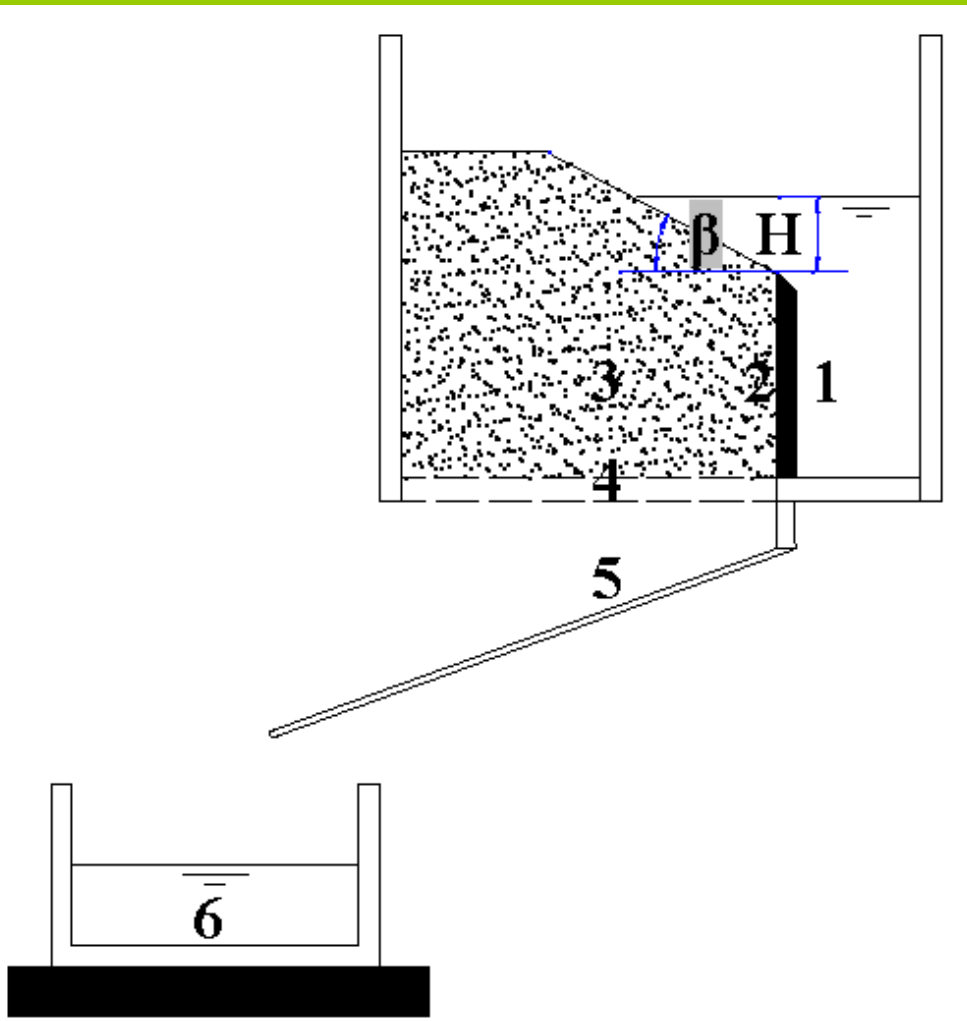
2 sand:

- medium size from 0.1 to 0.5 mm
- angle of the bank's slope from 10° to 30°.

impermeable floor valley (in the Bunjevac language called “mlaka” and “semlyék” in the Hungarian language)

LABORATORY SETUP

in a 9.4 cm wide laboratory channel



2: 16 cm high weir

3: sand (fractions of sizes 0.4-0.5 mm)

4: geotextile

5: channel

6: vessel, scale

At the beginning of the experiment, water level in sector 1 was equal to weir crest level, hence water depth above weir crest level was $H=0$.

By adding water up to $H_{max} \approx 7$ cm in sector 1, the water infiltrated through sector 3.

Water level in the sector 1 was measured by a grade with a millimeter accuracy.

At the same time, the water weight in the vessel on the scale and water density were measured and the mass diagram of the water volume infiltrated from sector 3, $V=f(t)$ was determined.

MEASUREMENT RESULTS

$\beta=12.3-12.6; 21.9-22.4$ and $27.9-28.5^\circ$

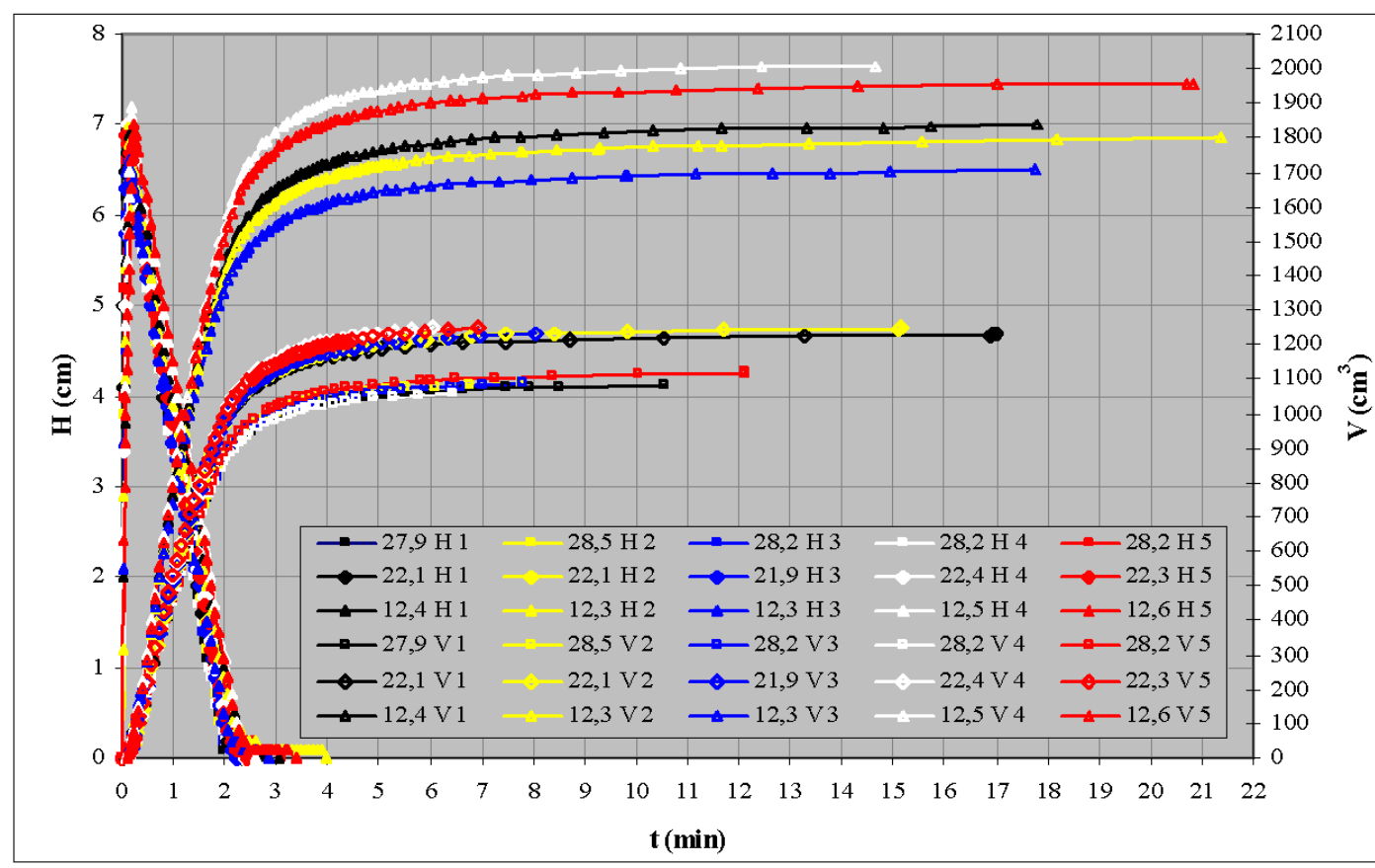
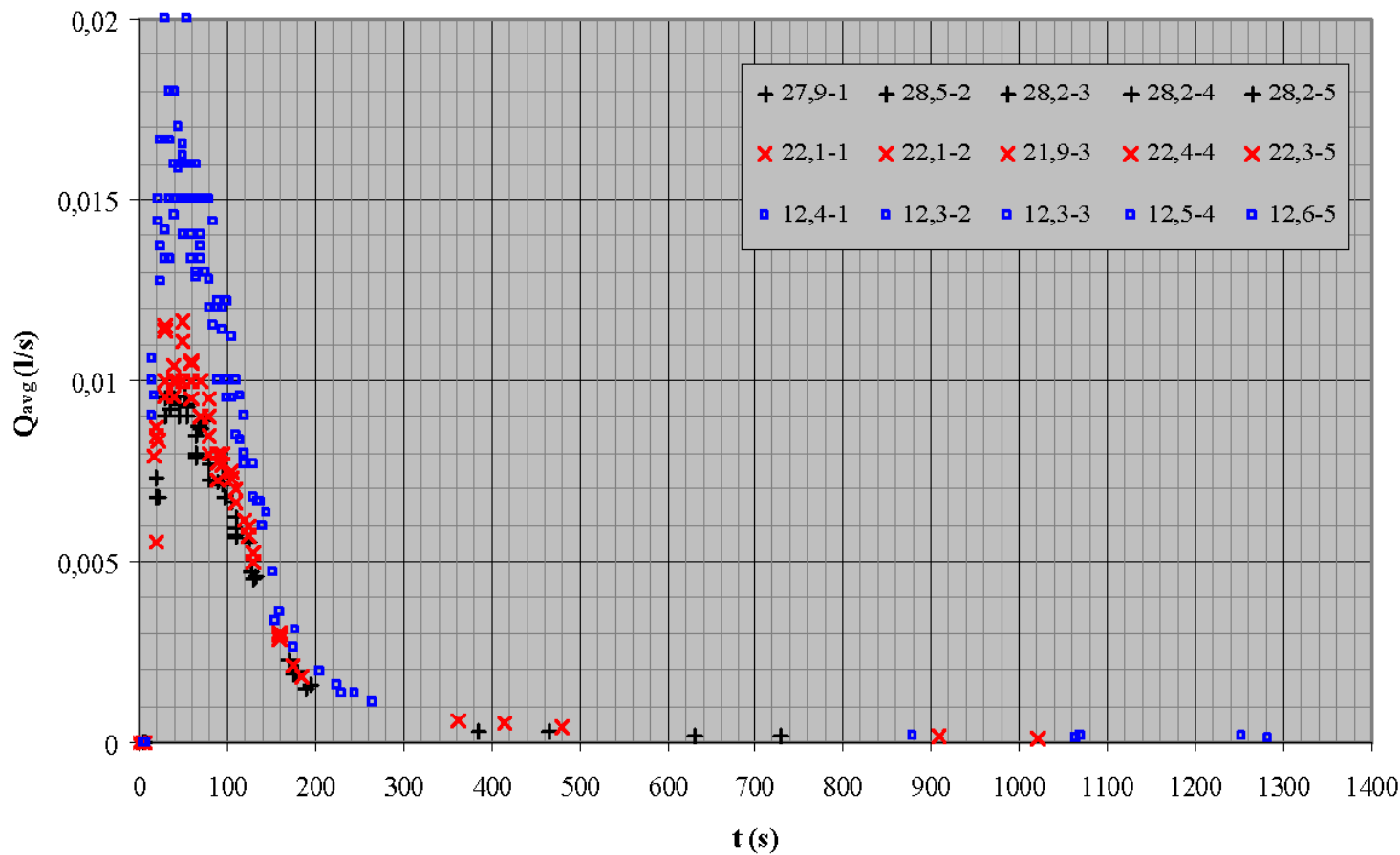


Diagram $H=f(t)$ for sector 1 and $V=f(t)$ for sector 3

The time of filling for sector 1 lasted 5-15 s, and the water started to overflow from sector 3 after 6-9 seconds, measured from the start of inflow into sector 1.

Hydrographs for sector 1: By increasing angle β , the mass diagram final value of the water volume infiltrated from sector 3 decreases.

$$Q_{avg} = \frac{\Delta V}{\Delta t}$$



Discharge hydrograph from the sector 3

From the start of water inflow into sector 1, the maximum discharge value (Q_{avg}) appeared at 29-55 seconds. Maximum discharge value (Q_{avg}) is decreased by the increase of angle β .

NUMERICAL MODEL FORMULATION

$$\frac{\partial \theta}{\partial t} = \frac{1}{J} \frac{\partial}{\partial \xi^i} \left(JK_e \frac{\partial \Pi}{\partial \xi^j} g^{ij} \right)$$

$$K_e = K_s K_r$$

$$p_c = \frac{\rho g}{\alpha} (S^{-1/m} - 1)^{1/n}$$

$$K_r = S^{1/2} \left\{ 1 - (1 - S^{1/m})^m \right\}^2$$

θ - volumetric water content

t - time

J - Jacobian of the transformation to Cartesian coordinates

ξ^i, ξ^j - coordinates in 'i' and 'j' direction

K_e - effective permeability

g_{ij} - metric tensor

Π - piezometric head

K_s - saturated hydraulic permeability of the soil

K_r - relative permeability

$p_c = p_a - p_w$ - capillary pressure

ρ - water density

g - gravitational acceleration

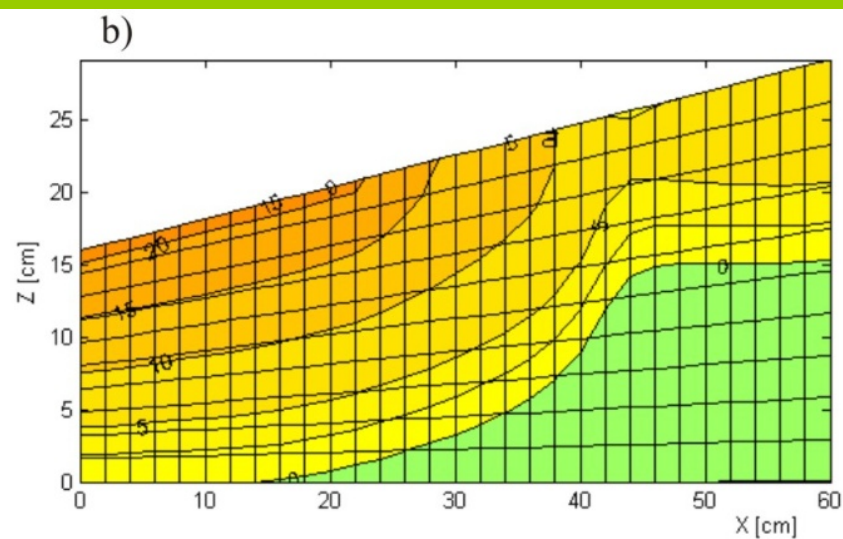
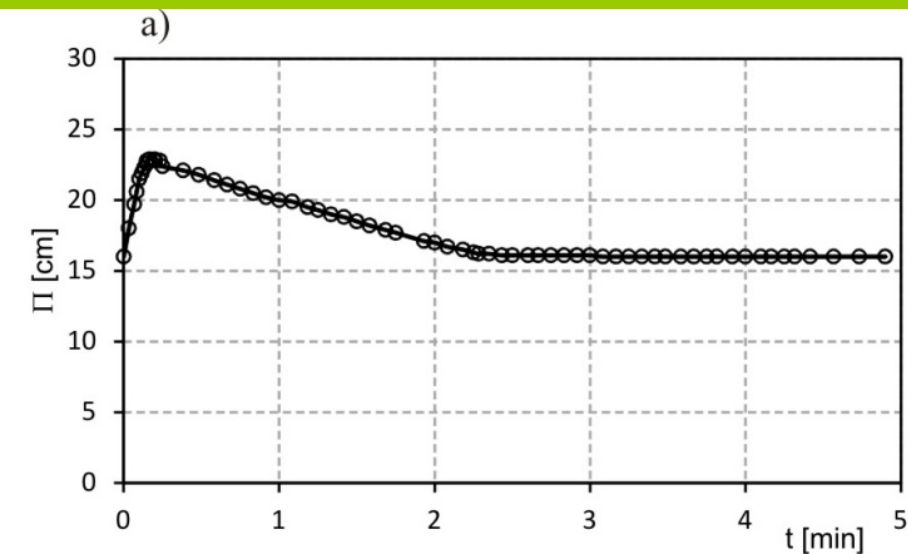
S - effective soil saturation

$m = 1 - 1/n$

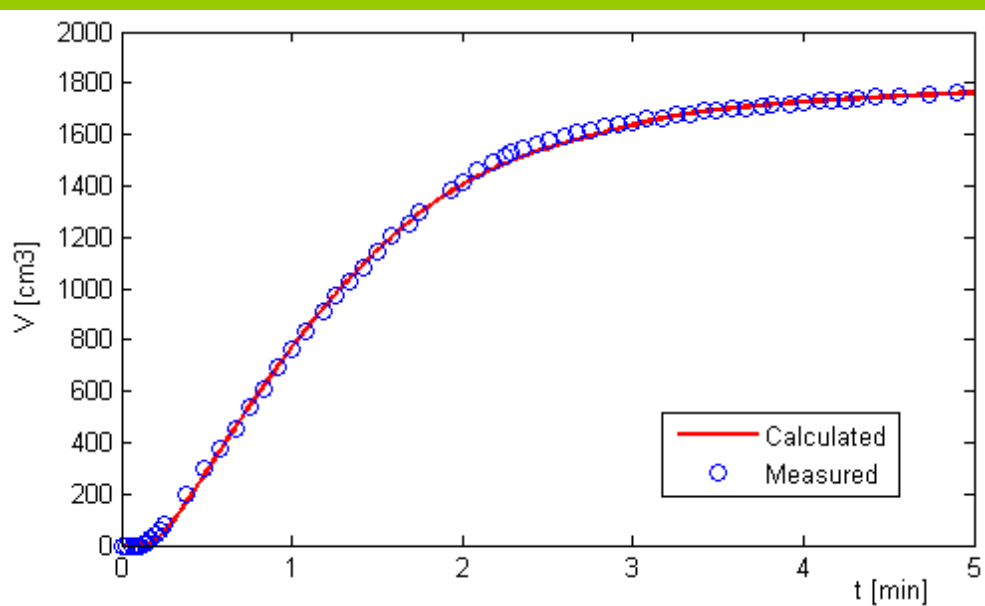
Obtained soil parameters in the process of model calibration

Parameter	Value
Hydraulic permeability	5.3×10^{-4} [m/s]
Porosity	0.21 [-]
Residual water content	0.09 [-]
α	12.2 [1/m]
n	2.5 [-]

RESULTS AND DISCUSSION



a) Upstream boundary water elevation during simulation, as observed during the experiment, b) calculated heads 40s after start of experiment/simulation



Comparison of calculated and measured cumulative water volumes at the downstream boundary.

